

RIGID PAVEMENT DESIGN  
(INTERIOR FLOOR SLABS)

PROJECT: \_\_\_\_\_

LOCATION: \_\_\_\_\_ DATE: \_\_\_\_\_

Design By: \_\_\_\_\_ Checked By: \_\_\_\_\_

SCOPE: Design of concrete floor slabs-on-grade subject to vehicular loads and to high stationary live loads (loads imposed by movable items).

REFERENCE: TM 5-809-12, Concrete Floor Slabs-on-Grade Subjected to Heavy Loads.  
TM 5-822-5, Pavement Design for Roads, Streets, Walks, and Open  
Storage Areas

***NOTE: The use of this form does not preclude compliance  
with all requirements of TM 5-809-12 or TM 5-822-5.***

I. NONREINFORCED STRENGTH DESIGN:

1. Traffic Volume(s):

a. Category of Traffic (page 3-1 & paragraph 3-2, Traffic Distribution).

b. <u>Category</u>	<u>Largest Maximum Axle Load (kips)*</u>	<u>Maximum Load Capacity (kips)</u>	<u>Maximum Operations/Day</u>
I	10	2 to 4	
II	15	4 to 6	
III	25	6 to 10	
IV	36	10 to 16	
V	43	16 to 20	
VI	120	20 to 52	

\*Subsequent tables and design curves use these values to represent each category.

Note: Data from Facilities Engineer's office and the Using Agency.

## 2. Design Index:

a. Categories I, II, III: Design index = \_\_\_\_ (page 5-1, Table 3, Traffic Categories for Design Index. This table good for all Cat. I and II forklifts, but only up to 5 passes of a Cat III forklift; i.e., 25-kip max axle-load forklift.)

### b. Categories III, IV, V, VI:

(1) No design indices: Separate curves for forklift max axle-loads of 25-kips and greater.

(2) Total design traffic: Total anticipated traffic volume is calculated for each max axle load forklift category over an anticipated 25-year life for the pavement.

\_\_\_\_-kip max axle load traffic = \_\_\_\_/day x \_\_\_\_ days/wk x \_\_\_\_ wks/yr x 25 yrs  
= \_\_\_\_\_ passes

\_\_\_\_-kip max axle load traffic = \_\_\_\_/day x \_\_\_\_ days/wk x \_\_\_\_ wks/yr x 25 yrs  
= \_\_\_\_\_ passes

\_\_\_\_-kip max axle load traffic = \_\_\_\_/day x \_\_\_\_ days/wk x \_\_\_\_ wks/yr x 25 yrs  
= \_\_\_\_\_ passes

\_\_\_\_-kip max axle load traffic = \_\_\_\_/day x \_\_\_\_ days/wk x \_\_\_\_ wks/yr x 25 yrs  
= \_\_\_\_\_ passes

3. Modulus of Subgrade Reaction (K) = \_\_\_\_ pci (Based on Field Plate Bearing Tests) or (Based on Table 2, TM 5-809-12 & Fig 9-1, TM 5-822-5.) (Equivalent K at top of \_\_\_\_-inch base course)

a. If test results are not available, refer to Table 2, Typical Values of Modulus of Subgrade Reaction, page 4-2, for typical values based on Unified Soils Classification System soil classification and on the in-situ moisture content of the soil. This yields a soil K-value = \_\_\_\_ psi.

b. Read para 5-3e, Non-uniformity (Subgrade), page 5-5: and para 5-5a, Subgrade Conditions (Steel Reinforcing), page 5-6..

c. When a base course is used, use Fig. 9-1, TM 5-822-5 (Jun 92) to determine the K-value on top of the base course; as a function of the base course thickness. This yields a K-value = \_\_\_\_ psi on top of the \_\_\_\_-inch base course.. It is good practice to run field plate bearing tests to confirm this value.

4. Concrete 28-day Flexural Strength (P) = \_\_\_\_ psi.

Based on compressive strength:  $P = (7.5 \text{ to } 10) \times \text{SQRT (Compressive Strength)}$

Ref: p.5, Design & Control of Conc Mixtures, PCA, 13th Edition

Use 7.5 to 8 for gravels and 9 to 10 for crushed stone.

$P = \_ \times \text{SQRT (____ psi)} = \_ \text{ psi.}$

5. Nonreinforced Pavement Thickness:

a. Theoretical nonreinforced pavement thickness ( $h_d$ ) = \_\_\_\_ inches (to nearest 0.1 inch).

(1) Categories I, II, III: Design index curves, Figure 5-1, Design Curves for Concrete Floor Slabs, page 5-2. These curves are good for all Cat. I and II forklifts, but only up to 5 passes of a Cat III forklift; i.e., 25-kip max axle-load forklift.). This yields  $h_d = \_ \text{ inches.}$

(2) Categories III, IV, V: Design curves by Axle Load & Passes, Figure 5-2, Design Curves for Concrete Floor Slabs for Heavy Forklifts, page 5-3. This yields  $h_d = \_ \text{ inches.}$

(3) Design is based on greatest thickness required from design curves for design index and for Category III, IV, and V traffic.

b. Nonreinforced pavement thickness = \_\_\_\_ inches.

*(NOTE: EIRS 78-04 Revision - Pavement Concrete. Thickness shall be expressed to the nearest whole or half inch. Round up midway values.)*

6. Steel Reinforcement for Nonreinforced Concrete Slabs (for odd-shaped slabs, for slabs with mismatched joints, and where non-uniform subgrade support is a concern in frost regions.).

a. No reduction is allowed in pavement thickness for this reinforcement.

b. Minimum 0.06% distributed steel required in both directions (paragraph 5-5, Steel Reinforcement, pages 5-6 & 5-8).

$.0006 \times \_ \text{ " } \times 12 \text{ " / } = \_ \text{ square inch steel per foot of concrete}$

USE " x " - W x W WWM yields: \_\_\_\_ square inch steel per foot of concrete  
(closest common stock size)

Maximum Slab Length (Maximum Joint Spacing):

$$L = \{0.00047 h_r (f_s S)^2\}^{1/3} \text{ Ref Eqn 5-2, para 5-7a(3)d, page 5-16, TM 5-809-12}$$

$L = \underline{\hspace{1cm}}$  ft say  $\underline{\hspace{1cm}}$  ft      where:  $h_r = \underline{\hspace{1cm}}$  " (reinf slab thickness)  
 $f_s = 56,000$  psi (steel yield strength)  
 $S = \%$  reinf steel  
 $= (\underline{\hspace{1cm}} \text{ sq in per ft} / \underline{\hspace{1cm}} \text{ " x 12"/}) 100\%$   
 $= \underline{\hspace{1cm}} \%$

**II. REINFORCED STRENGTH DESIGN:**

1. Purpose: To increase the size of floor slab panels between joints or to decrease slab thickness requirements.

2. Graphic solution (nomogram):

a. Required thickness of nonreinforced floor slab to nearest 0.1 inch =  $\underline{\hspace{1cm}}$  inches ( $h_d$ ) (from step I5).

b. Desired thickness of reinforced floor slab =  $\underline{\hspace{1cm}}$  inches ( $h_r$ ) (6-inch minimum).

c. Nomogram: Page 5-9, Figure 5-4, Design Thickness for Reinforced Floor Slabs.

(1) Percent steel required =  $\underline{\hspace{1cm}}$  (S, %)

(2) Maximum allowable length of reinforced pavement slabs =  $\underline{\hspace{1cm}}$  feet (L) (25 feet minimum, 75 feet maximum).

$(S\%/100) \times h_r \text{ " x 12"/} = \text{No. of square inches steel required per foot of concrete}$   
 $\underline{\hspace{1cm}} \times \underline{\hspace{1cm}} \text{ " x 12"/} = \underline{\hspace{1cm}} \text{ square inch steel per foot of concrete}$

**USE  $\underline{\hspace{1cm}}$  " x  $\underline{\hspace{1cm}}$  " - W  $\underline{\hspace{1cm}}$  x W  $\underline{\hspace{1cm}}$  WWM** yields:  $\underline{\hspace{1cm}}$  square inch steel per foot of concrete (closest common stock size)

Maximum Slab Length (Maximum Joint Spacing):

$$L = \{0.00047 h_r (f_s S)^2\}^{1/3} \text{ Ref Eqn 5-2, para 5-7a(3)d, page 5-16, TM 5-809-12}$$

$L = \underline{\hspace{1cm}}$  ft say  $\underline{\hspace{1cm}}$  ft      where:  $h_r = \underline{\hspace{1cm}}$  " (reinf slab thickness)  
 $f_s = 56,000$  psi (steel yield strength)  
 $S = \%$  reinf steel  
 $= (\underline{\hspace{1cm}} \text{ sq in per ft} / \underline{\hspace{1cm}} \text{ " x 12"/}) 100\%$   
 $= \underline{\hspace{1cm}} \%$

### III. FINAL PAVEMENT SECTION:

#### 1. Interior Concrete Pavement Section:

\_\_\_\_ " Concrete (\_\_\_\_ psi flexural strength @ 28 days)  
with \_\_\_\_" x \_\_\_\_" - **W\_\_\_\_ x W\_\_\_\_ WWM** placed \_\_\_\_" below the concrete surface.  
6-mil polyethylene sheeting (Vapor Barrier)  
\_\_\_\_" RDM (#57) Base Course { AASHTO M43, #57 crushed stone }

Maximum allowable length of reinforced pavement slabs = \_\_\_\_ feet (L) (25 feet minimum, 75 feet maximum).

2. Reinforcing Around Bollards, Columns and Other Projections Through Pavement Surface: To minimize cracking around projections and to hold these cracks together, place the following additional reinforcing at the corners of all projections, with a minimum of four sets of bars around circular projections greater than 6 inches in diameter:

**2 No. 4 Reinforcing Bars, each 4 feet long, Spaced 4 inches apart.**  
**Place bars at the mid-point of the slab**

3. Dowel Size and Spacing: (Ref Table 5-3, Dowel Size and Spacing, page 5-23)

**\_\_\_\_-inch Dia. Bar, \_\_\_\_ inches long (min)**  
**Spaced \_\_\_\_ inches (min) center to center.**

### IV. MAXIMUM ALLOWABLE STATIONARY LIVE LOAD:

1. Concrete floor slab thickness = \_\_\_\_ inches (from Step III-1a).
2. 28-day Flexural strength of concrete (P) = \_\_\_\_ psi. (from step I-4)
3. Stationary live load = \_\_\_\_ psf (Table 3-1, Maximum Allowable Stationary Live Load, page 3-3).
4. Modulus of Subgrade Reaction (K) = \_\_\_\_ pci. (from step I-3)
5. Constant factor = \_\_\_\_ (Table 3-1, page 3-3).
6. Maximum stationary live load = \_\_\_\_ x \_\_\_\_ = \_\_\_\_ psf  
(Step IV3) x (Step IV5)

**Note:** The potential consolidation of underlying soils shall be evaluated to determine that no detrimental settlements will occur before specifying stationary live loads greater than 500 psf.